



OPEN DAY ON DAMS: Verifiche, Controlli, Adeguamenti
Università degli Studi di Napoli Federico II – 10 Aprile 2024



Assessment of seismic performance and fragility of a CFRD: Workflow for Potrerillos dam in Mendoza (Argentina)



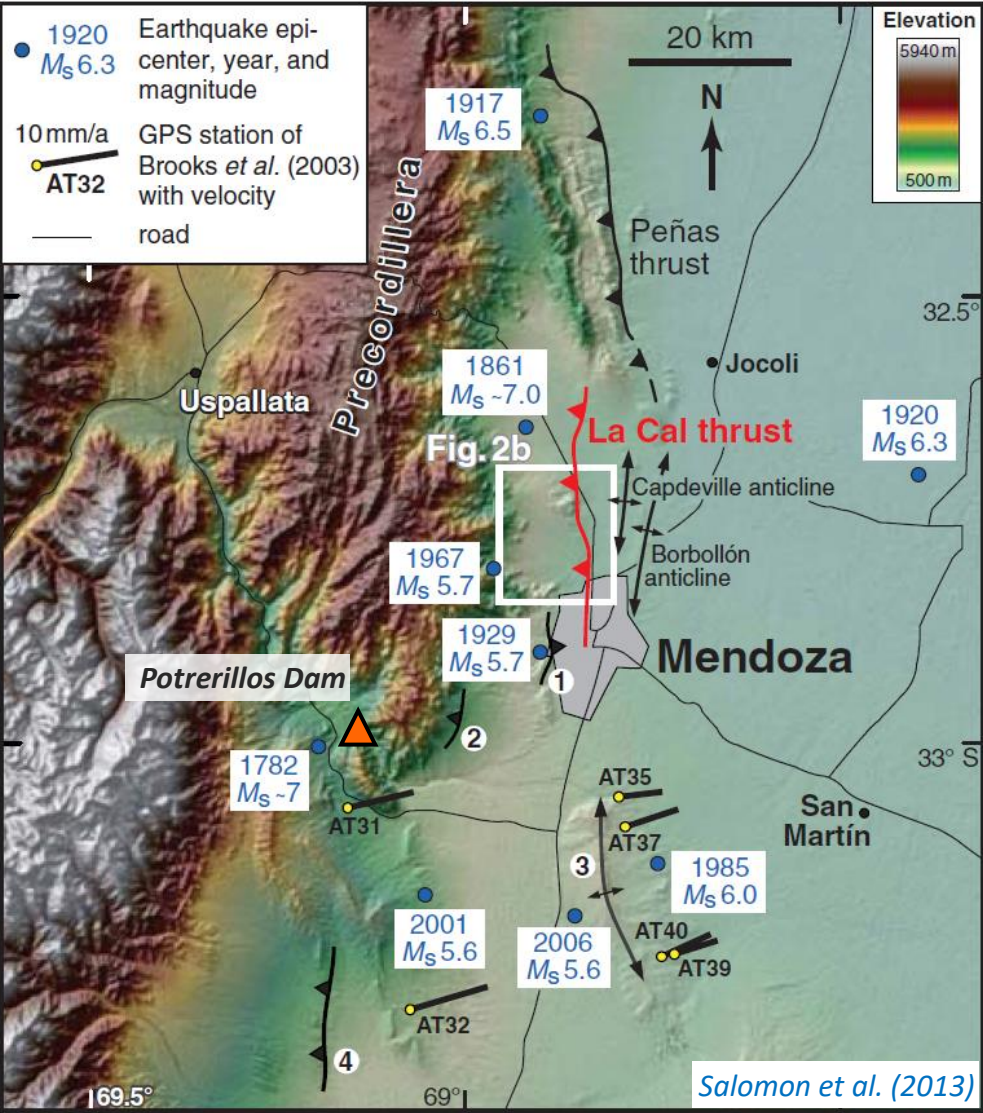
UNIVERSITÀ DEGLI STUDI DI NAPOLI
FEDERICO II

Juan Manuel Barbagelata

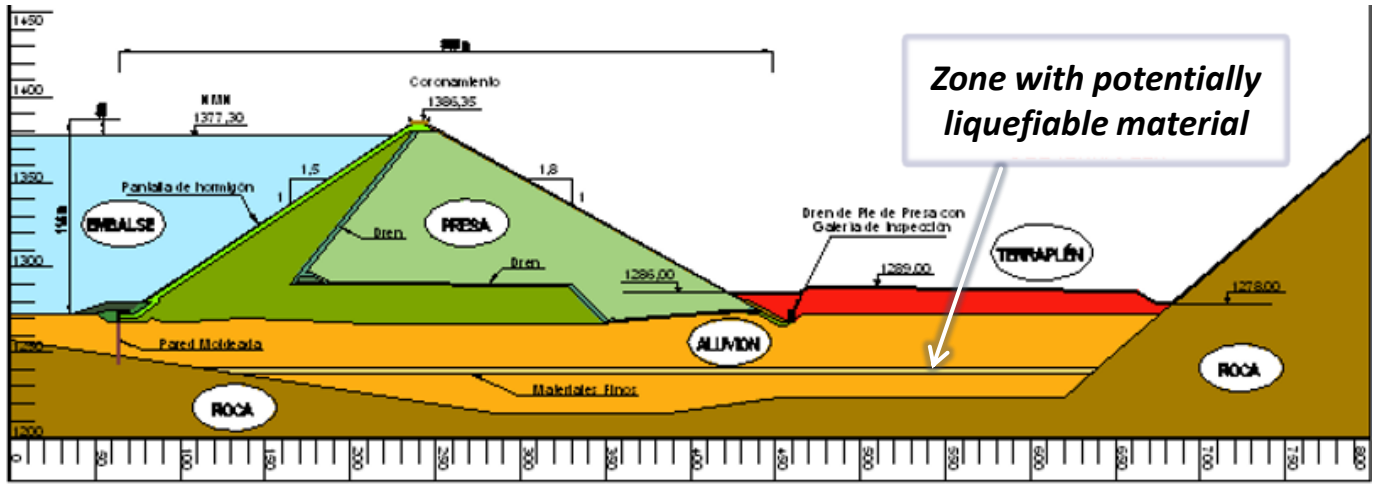
Ph.D. Candidate, DICEA, UniNa

Advisors: F. Silvestri, A. d'Onofrio & L. Pagano

Largest earthquakes and main active structures in the vicinity



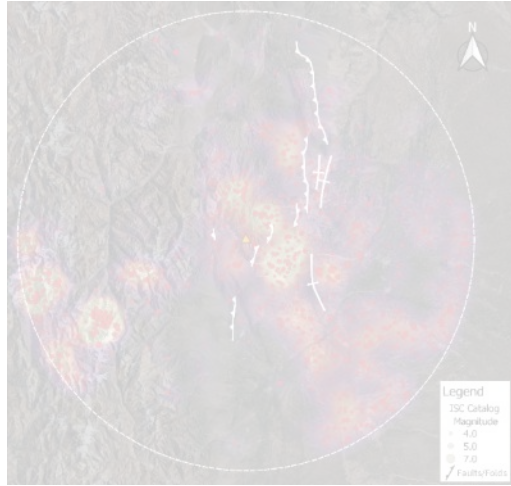
Sand and silty-sand lenses found in the alluvium during construction stage



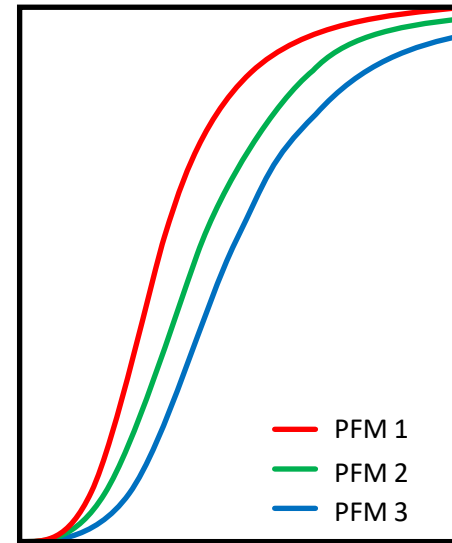
Stabilizing embankment downstream the dam as a countermeasure



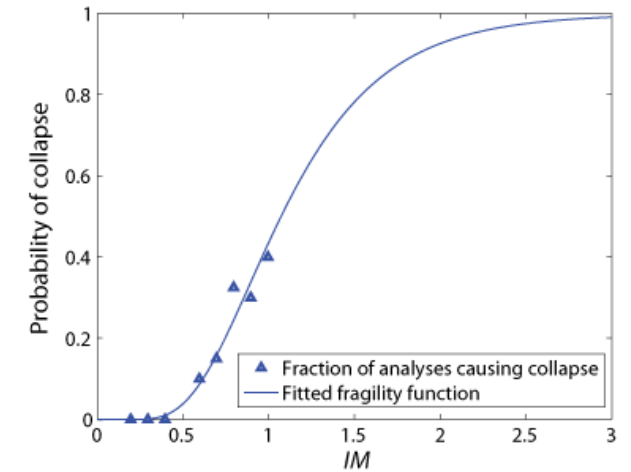
*Site-specific seismic hazard analysis
and record selection*



*Dam-specific Fragility
Functions*

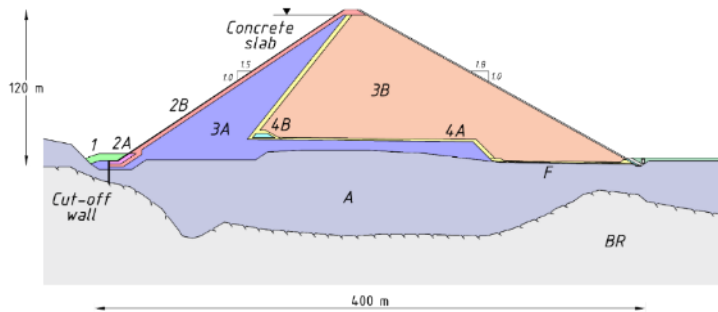
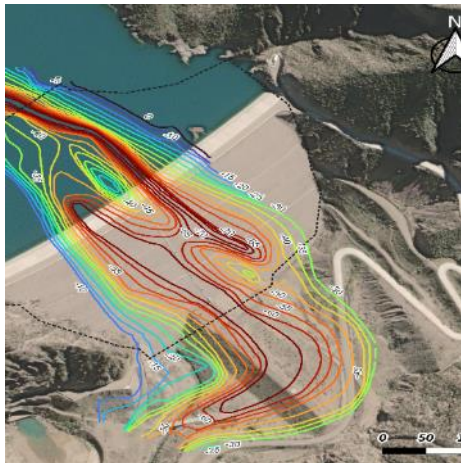


Fragility functions fitting

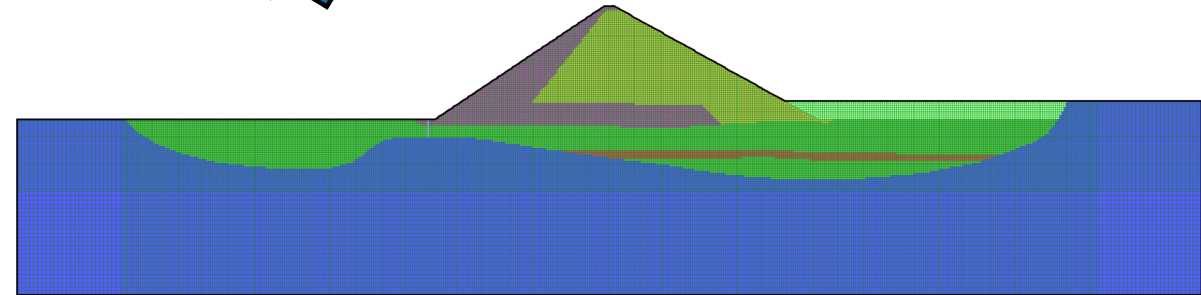


Baker (2015)

Geological-Geotechnical model



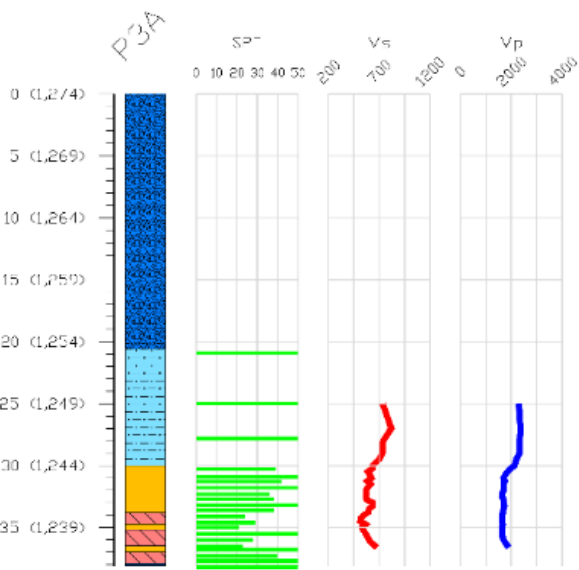
Advanced Numerical Model



Geometrical and geotechnical characterization

Field surveys and in-situ tests

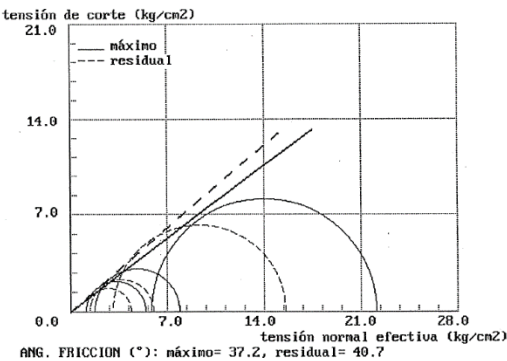
Boreholes, SPT and Cross-hole



Laboratory tests

Static triaxial test – TX CU

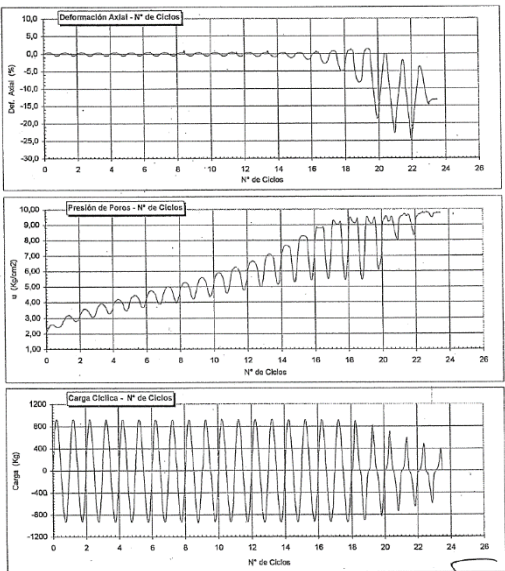
ENSAYOS DE COMPRESION TRIAXIAL
material: POTRERILLOS - muestra CF6
probetas $\phi=17\text{cm}$ - remoldeadas bajo $1\frac{1}{2}$ " - den. 2.061 Saturada
ENSAYOS CONSOLIDADOS NO DRENADOS



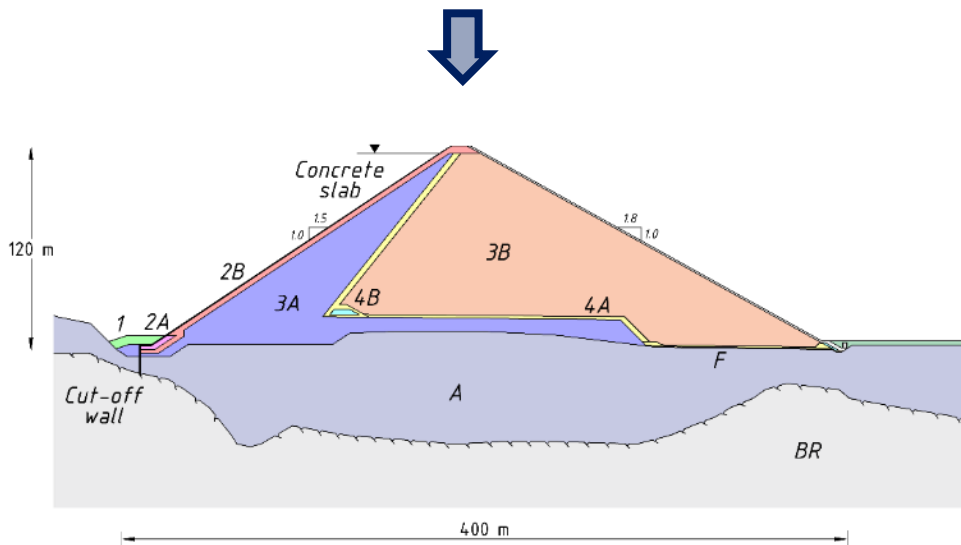
Real conditions at the dam site



Cyclic triaxial test – CTX



Geometry and material zoning for analysis

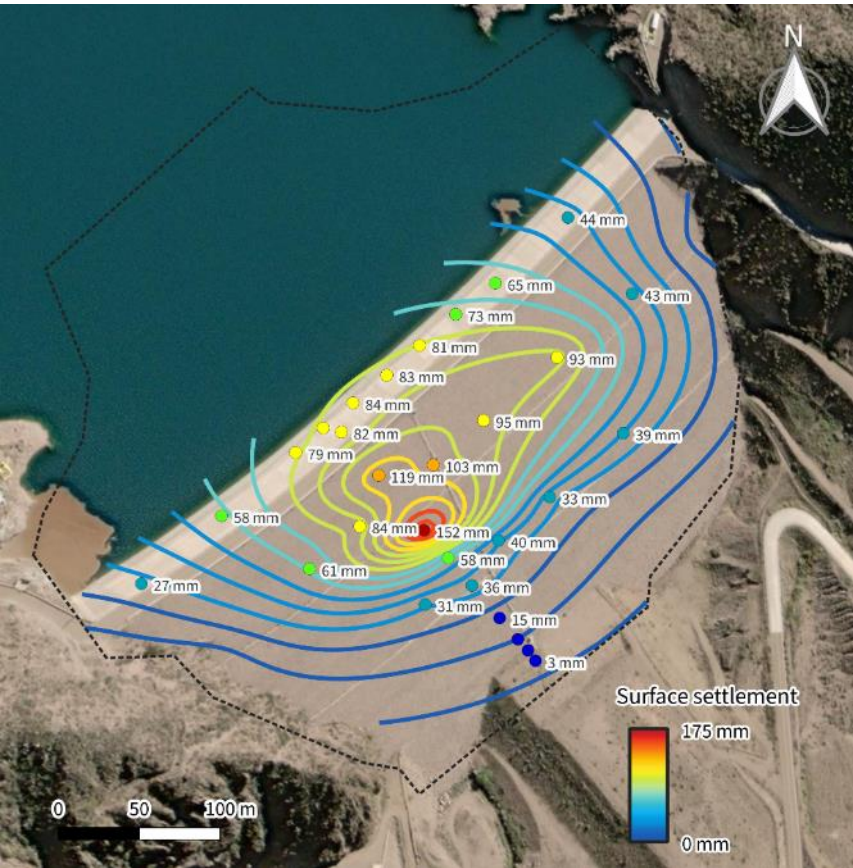


Geometrical and geotechnical characterization



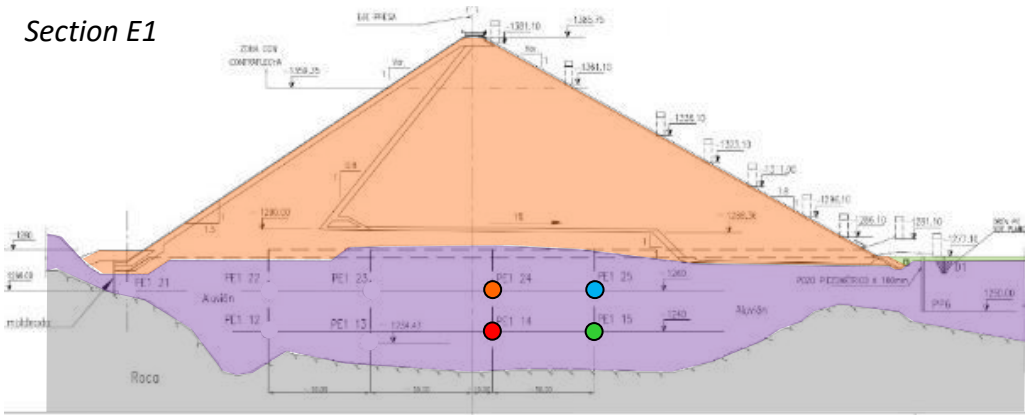
History of loads and hydraulic conditions

Topographic survey of surface settlement

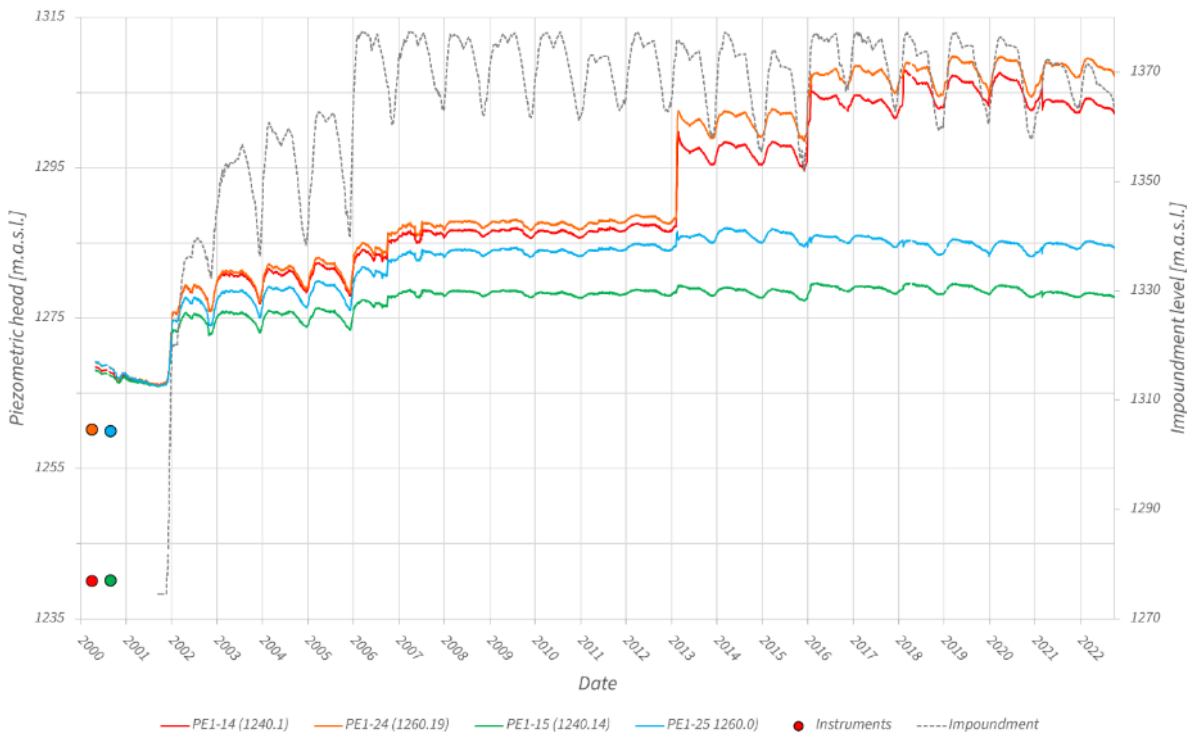


Analysis of pore water pressure records from piezometers

Section E1



PWP & impoundment level vs. time



Geometrical and geotechnical characterization

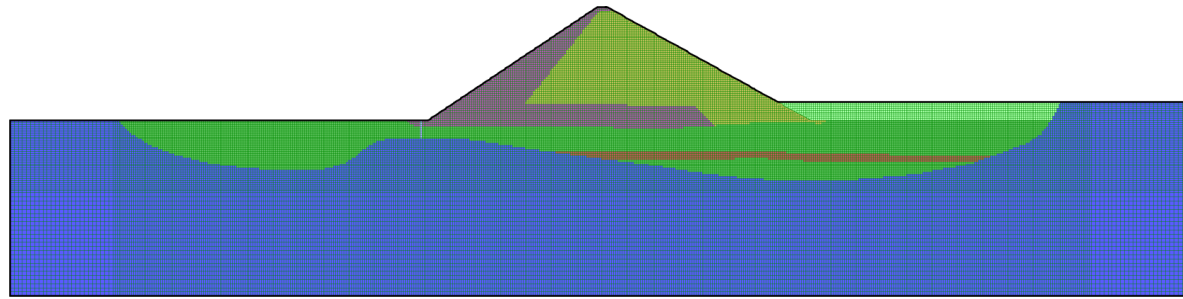


History of loads and hydraulic conditions



Definition of the numerical models

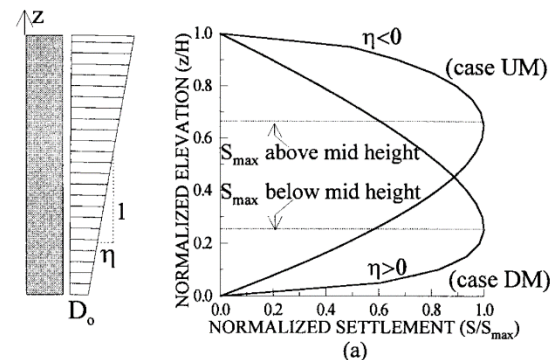
Calibration



Finite Difference Model (FLAC 8.0)

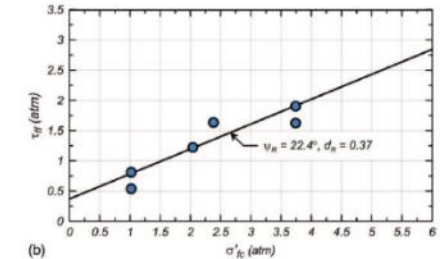
- Selection of 2D sections
- Definition of soil regions
- Discretization
- Boundary conditions
 - Mechanical
 - Hydraulic

Vertical stiffness by back-analysis of settlement profiles



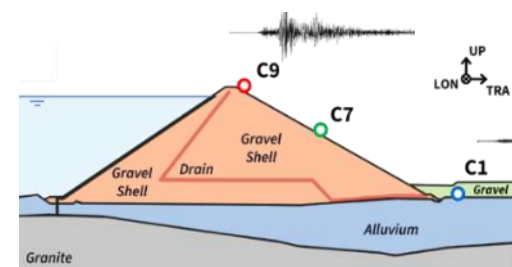
Pagano et al. (1998)

Calibration of material models using laboratory tests

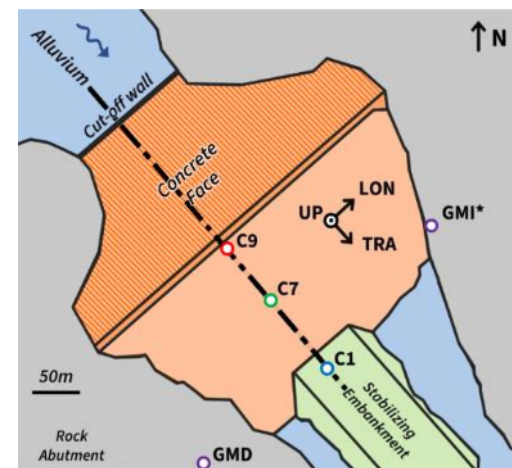
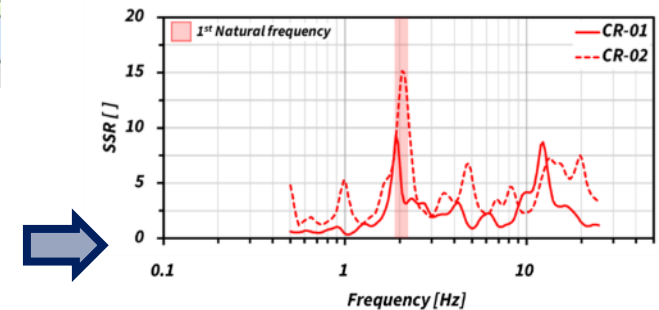


Boulanger (2019)

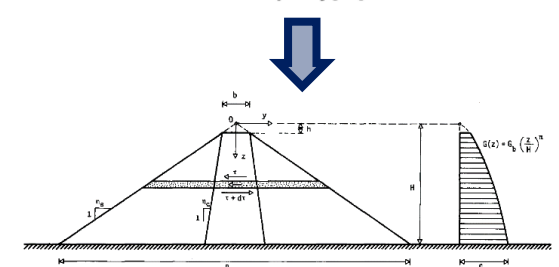
Calibration of global parameters



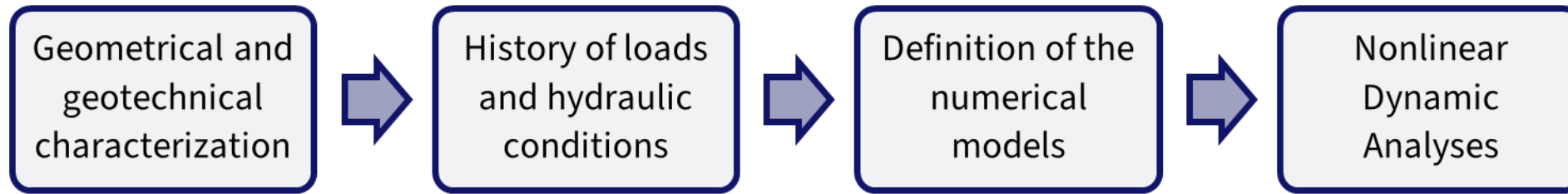
Natural frequencies



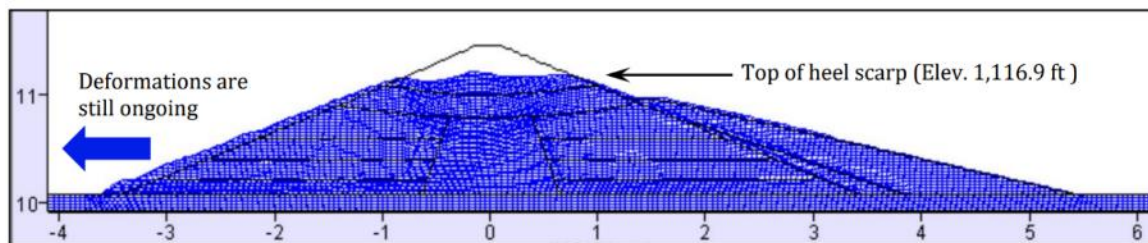
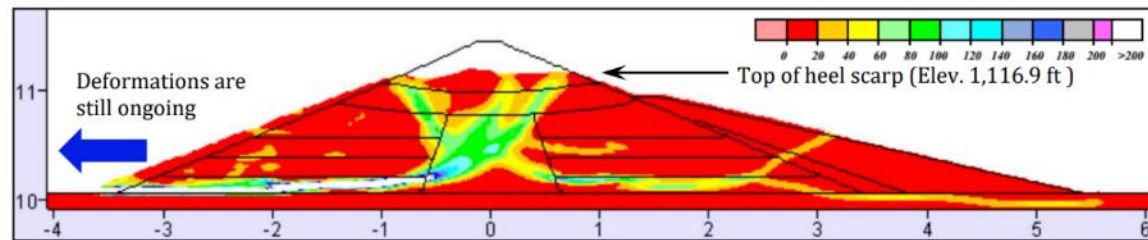
Barbagelata et al. (2024)



Dakoulas & Gazetas (1985)

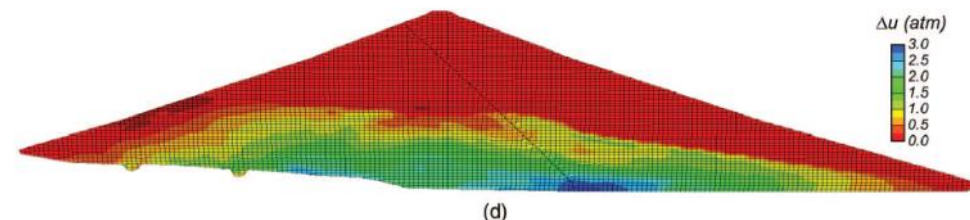
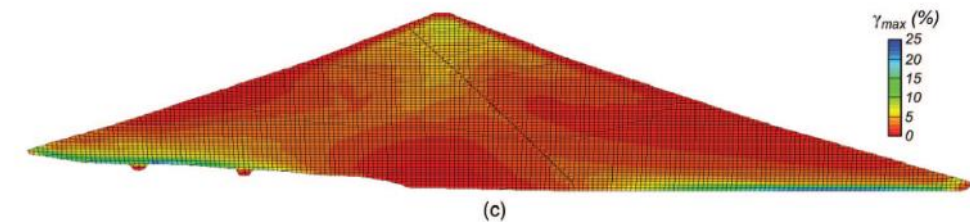
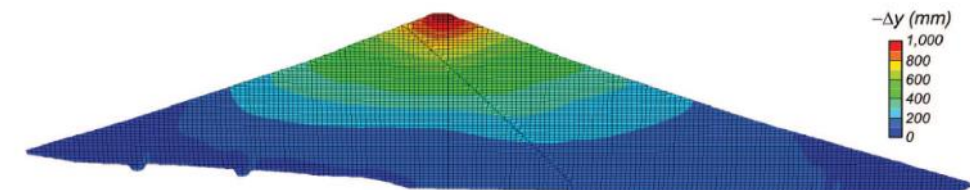
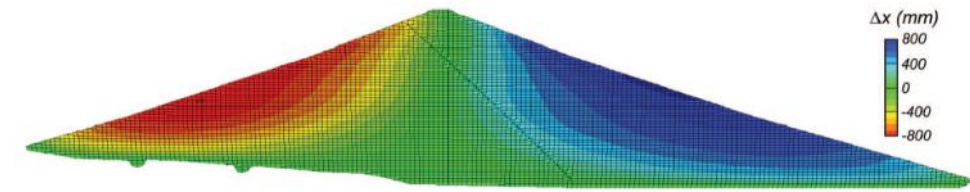


Output examples from NDAs in embankment dams

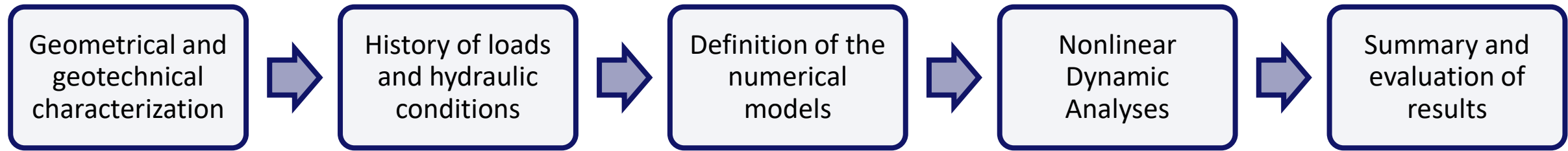


Lower San Fernando Dam (Chowdhury et al., 2019)

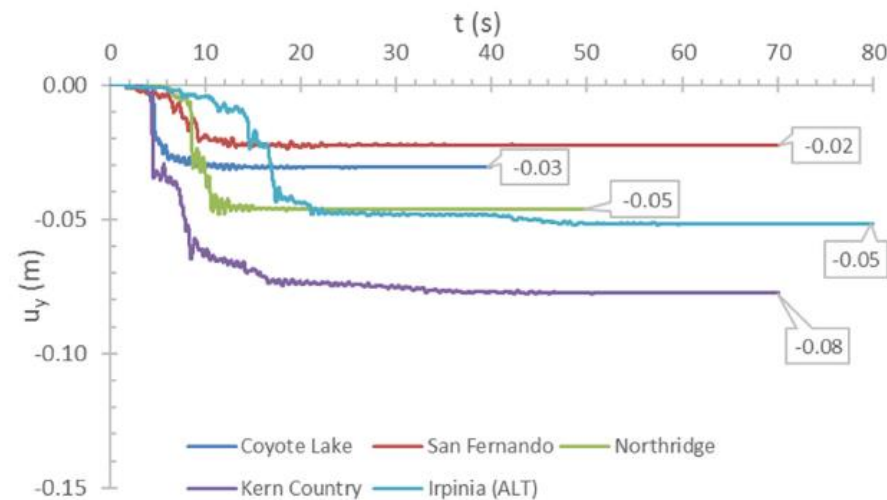
The models are able to capture the complex response of the dam during strong ground motions



Austrian Dam (Boulanger, 2019)

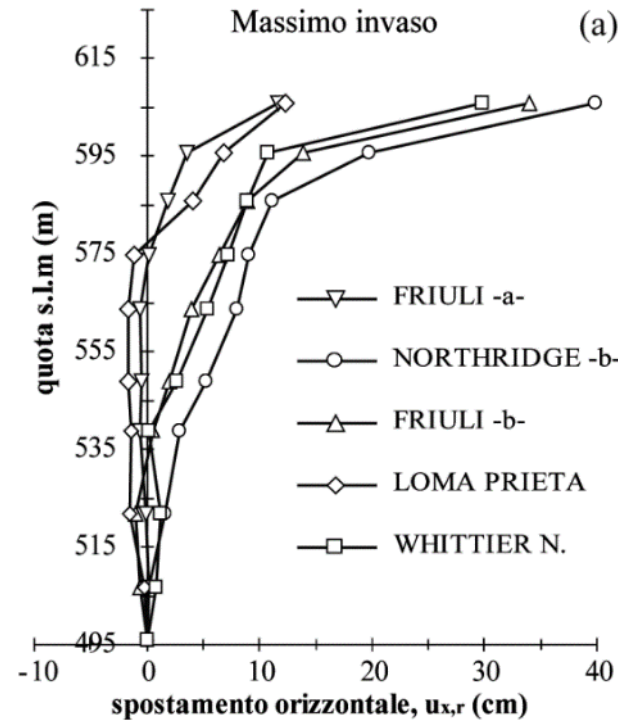


Time histories of vertical settlements at the crest of the dam



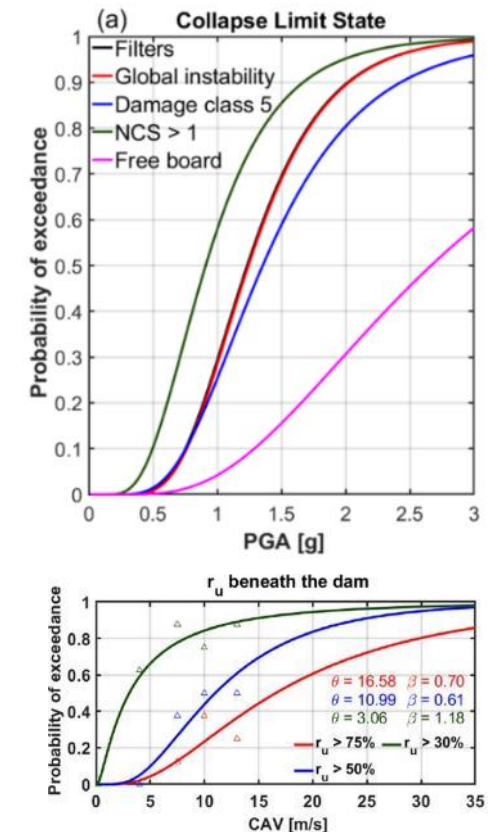
Monte Cotugno rockfill dam
(Costigliola, 2017)

Horizontal residual displacements at the center line of the dam



Melito rockfill dam
(Costanzo et al., 2011)

Fragility functions for different Potential Failure Modes (PFMs)



Farneto zoned dam
(Regina et al., 2022)

1 – Filling material - Silt

- Granulometric analyses

2B - Transition gravel

- Granulometric analyses
- Dry unit weight (during constr.)
- Laboratory permeability

2A/F – Clean sand filter

- Granulometric analyses
- Laboratory permeability
- In-situ permeability (Test Embk.)

4A/4B - Clean gravel drain

- Granulometric analyses
- In-situ permeability (Test Embk.)
- Dry unit weight (Test Embk.)
- Load-plate tests (Test Embk.)

BR – Bedrock - Granite

- Unconfined compression test
- Ultrasonic tests (sample)

3A - Coarse gravel shell

- Granulometric analyses
- Max and Min density
- Laboratory permeability
- In-situ permeability (Test Embk.)
- Load-plate tests (Test Embk.)
- TX CD Ø 205 mm (d_{\max} 20 mm)
- Dry unit weight (during constr.)

Less variability in properties

3B - Coarse gravel shell

- Granulometric analyses
- Max and Min density
- Laboratory permeability
- In-situ permeability (Test Embk.)
- Load-plate tests (Test Embk.)
- TX CU Ø 75 mm (d_{\max} 12.7 mm)
- TX CD Ø 205 mm (d_{\max} 20 mm)
- Dry unit weight (during constr.)

E –Stabilizing embankment

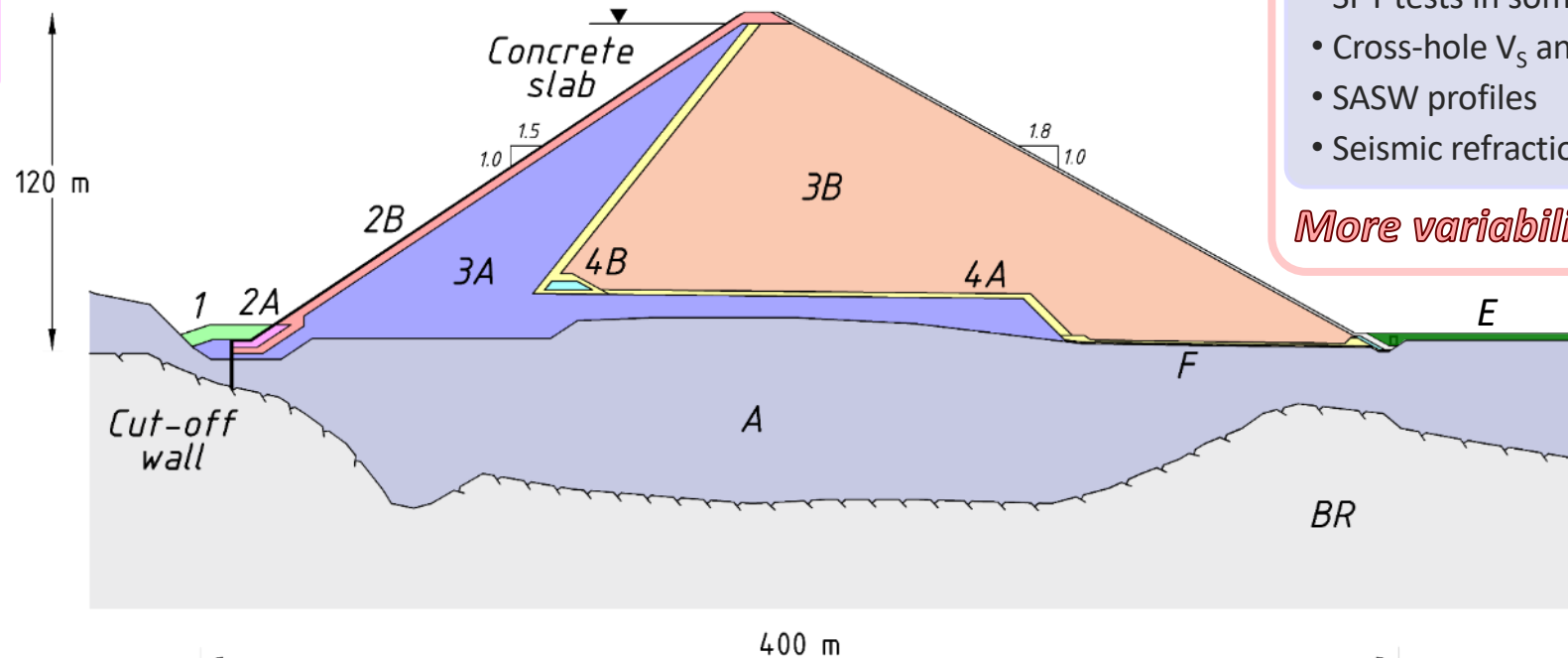
- Same coarse gravel as 3B

A - River alluvium

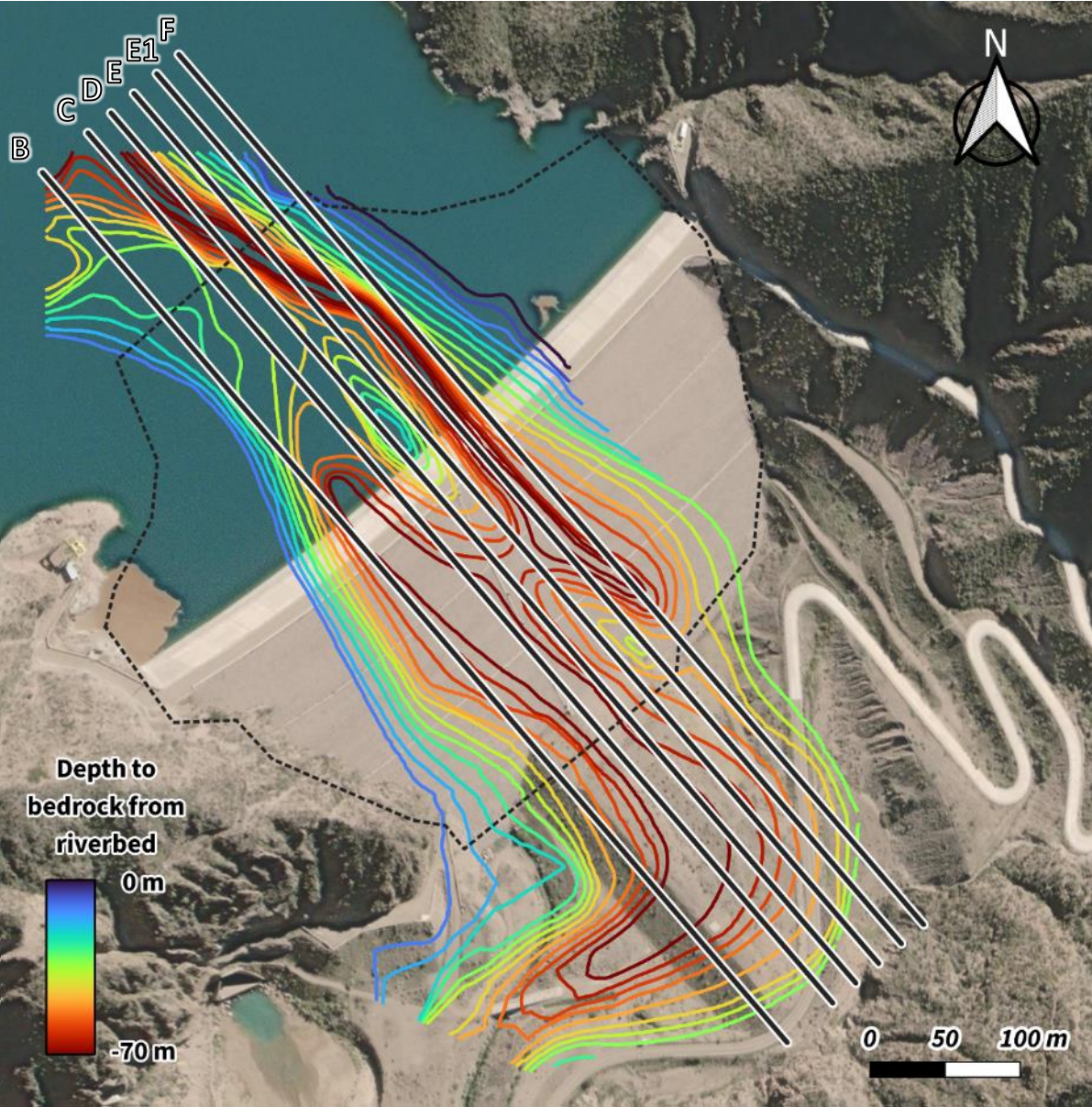
- Shallow test pits
- Granulometric analyses
- Laboratory permeability
- TX CD Ø 170 mm (d_{\max} 38 mm)
- TX CU Ø 170 mm (d_{\max} 38 mm)
- CTX Ø 180 mm (d_{\max} 38 mm)
- Boreholes w/log profile
- SPT tests in some boreholes
- Cross-hole V_s and V_p profiles
- SASW profiles
- Seismic refraction sections

More variability in properties

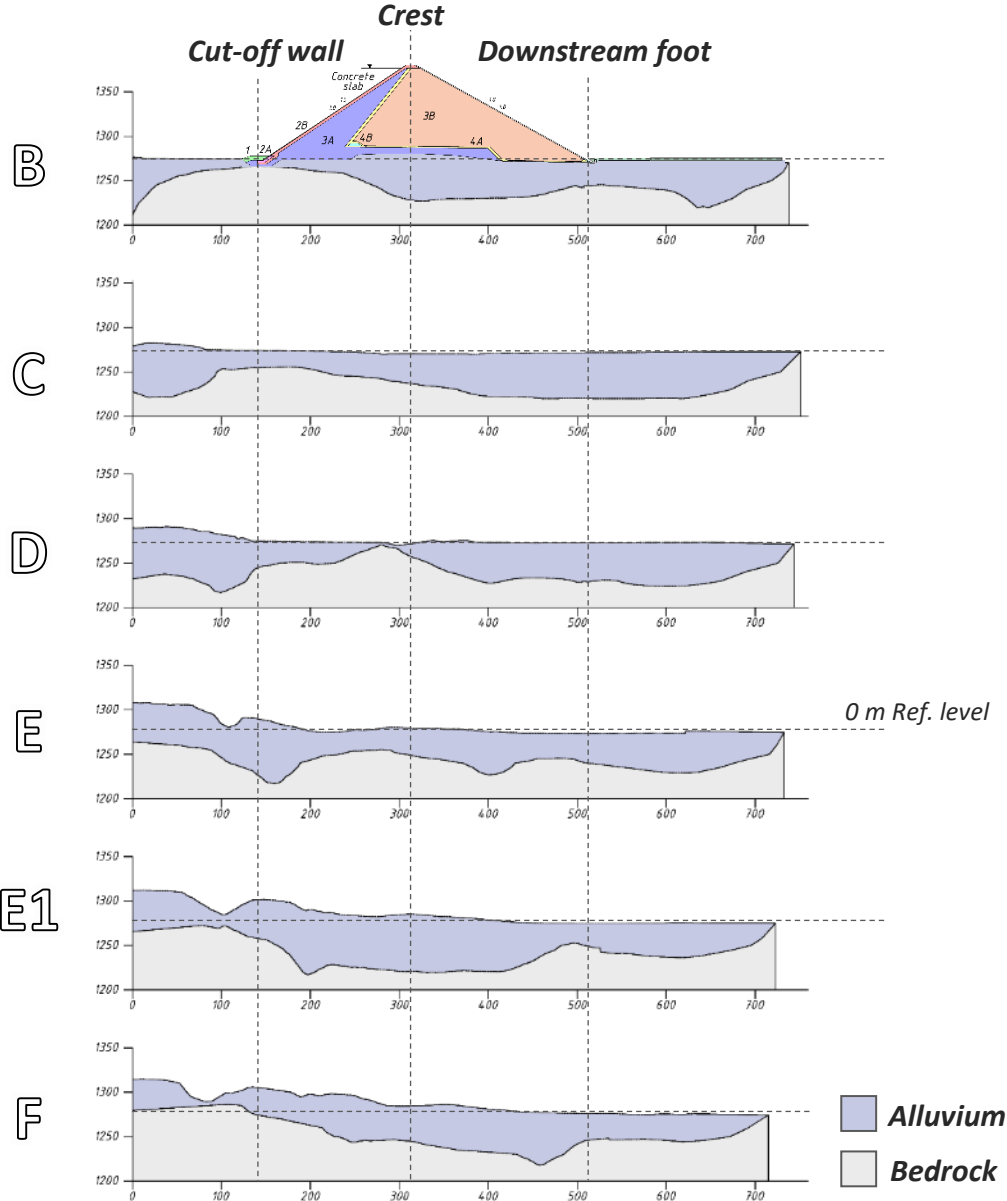
Center-left section of the dam

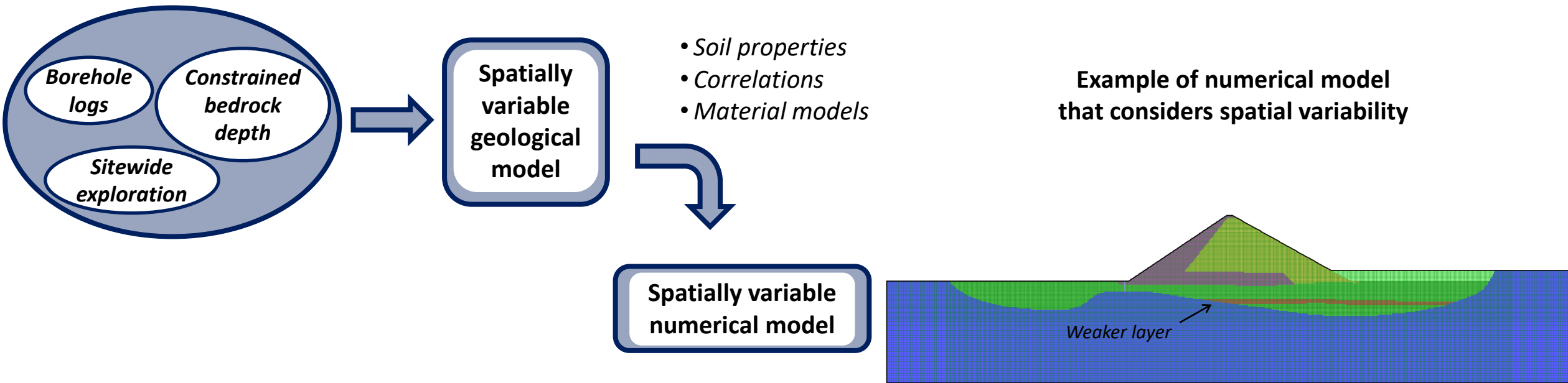


Contours of bedrock depth under dam foundation level

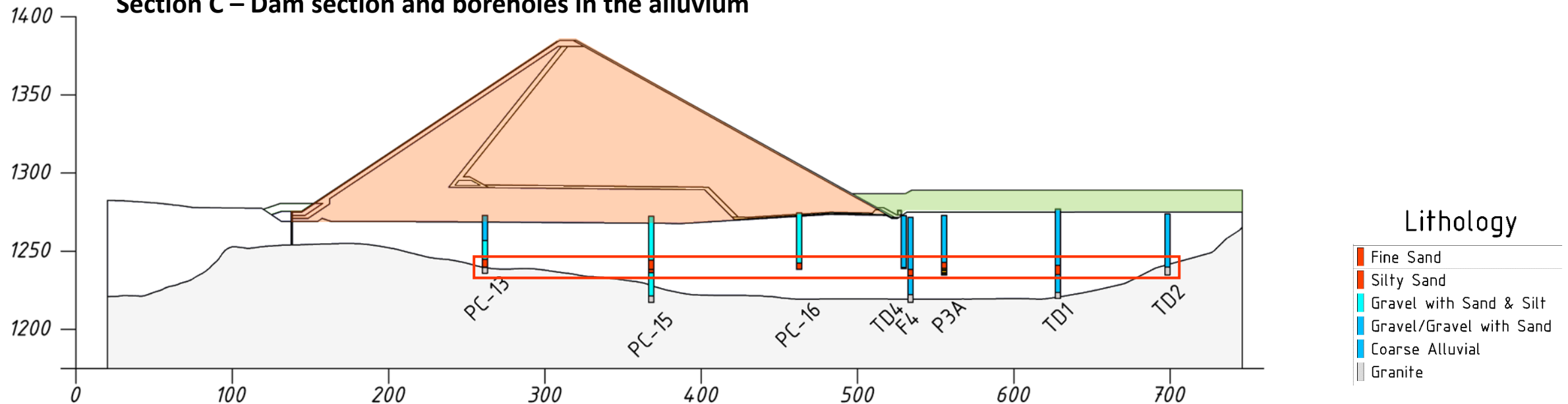


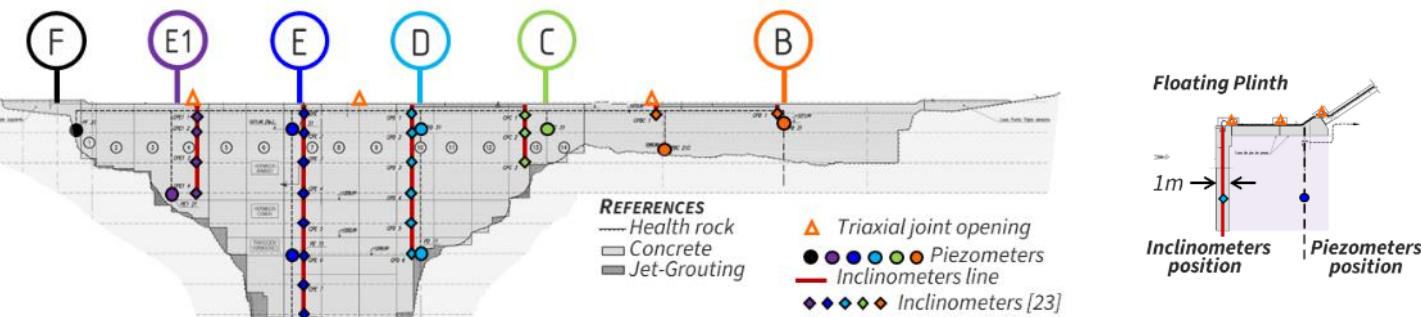
Cross sections



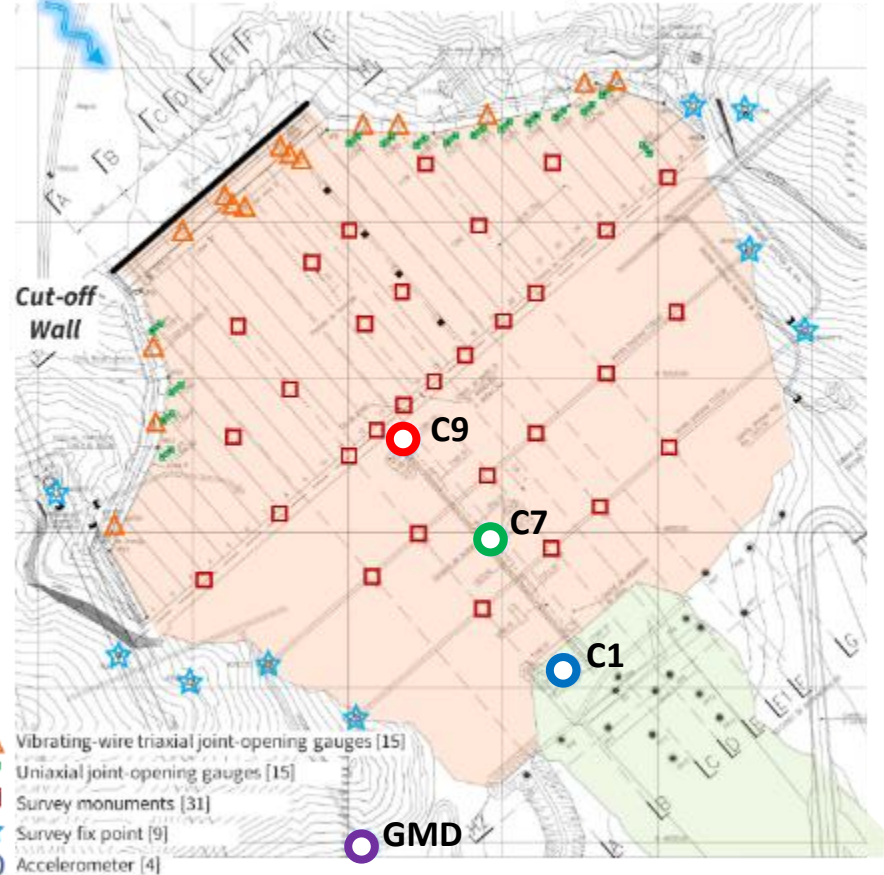


Section C – Dam section and boreholes in the alluvium

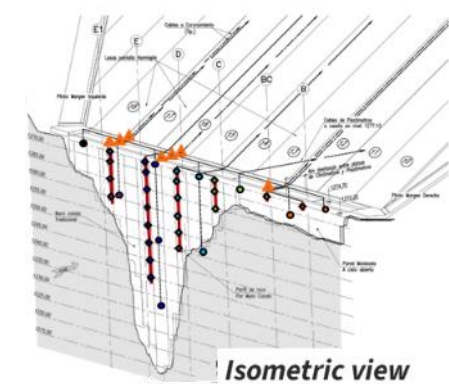




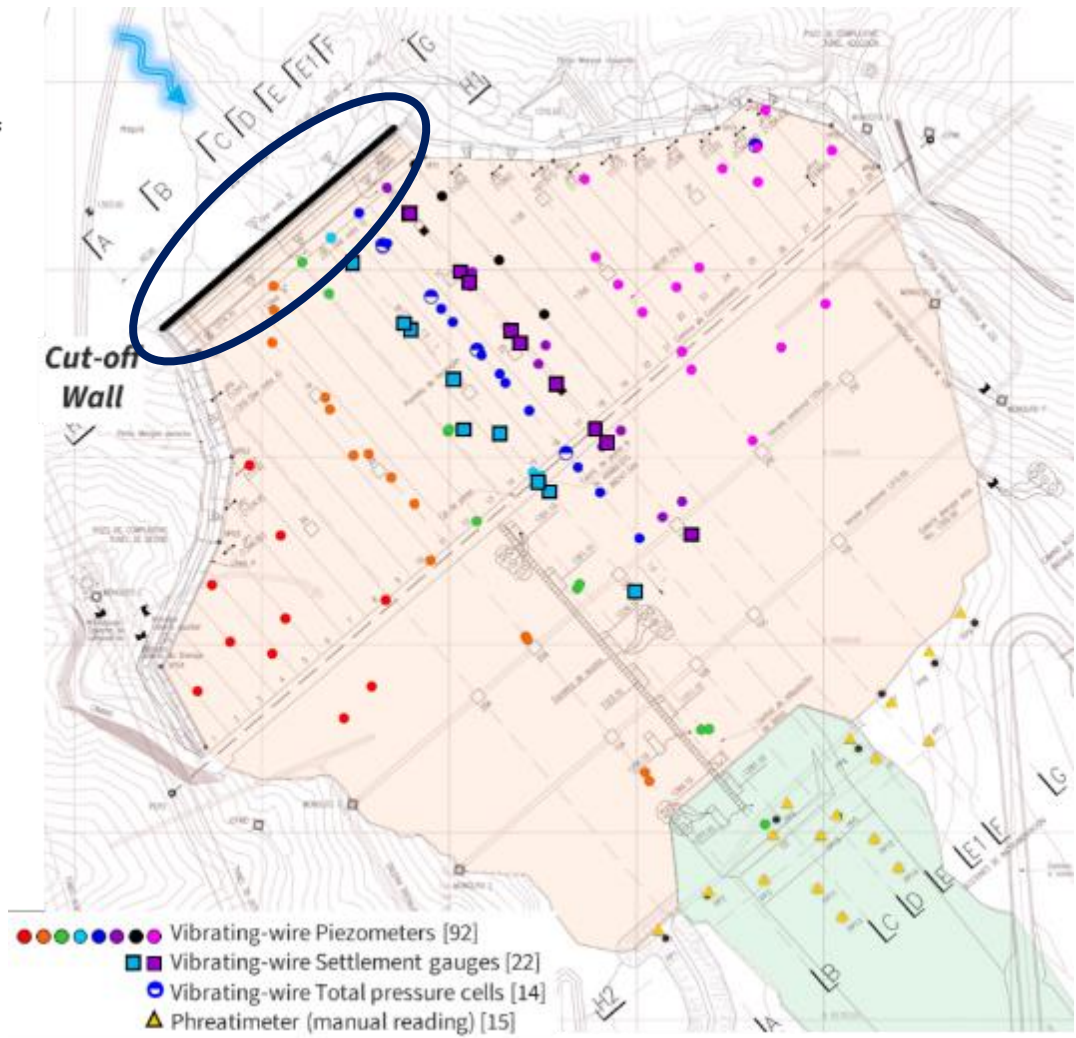
Surficial instruments



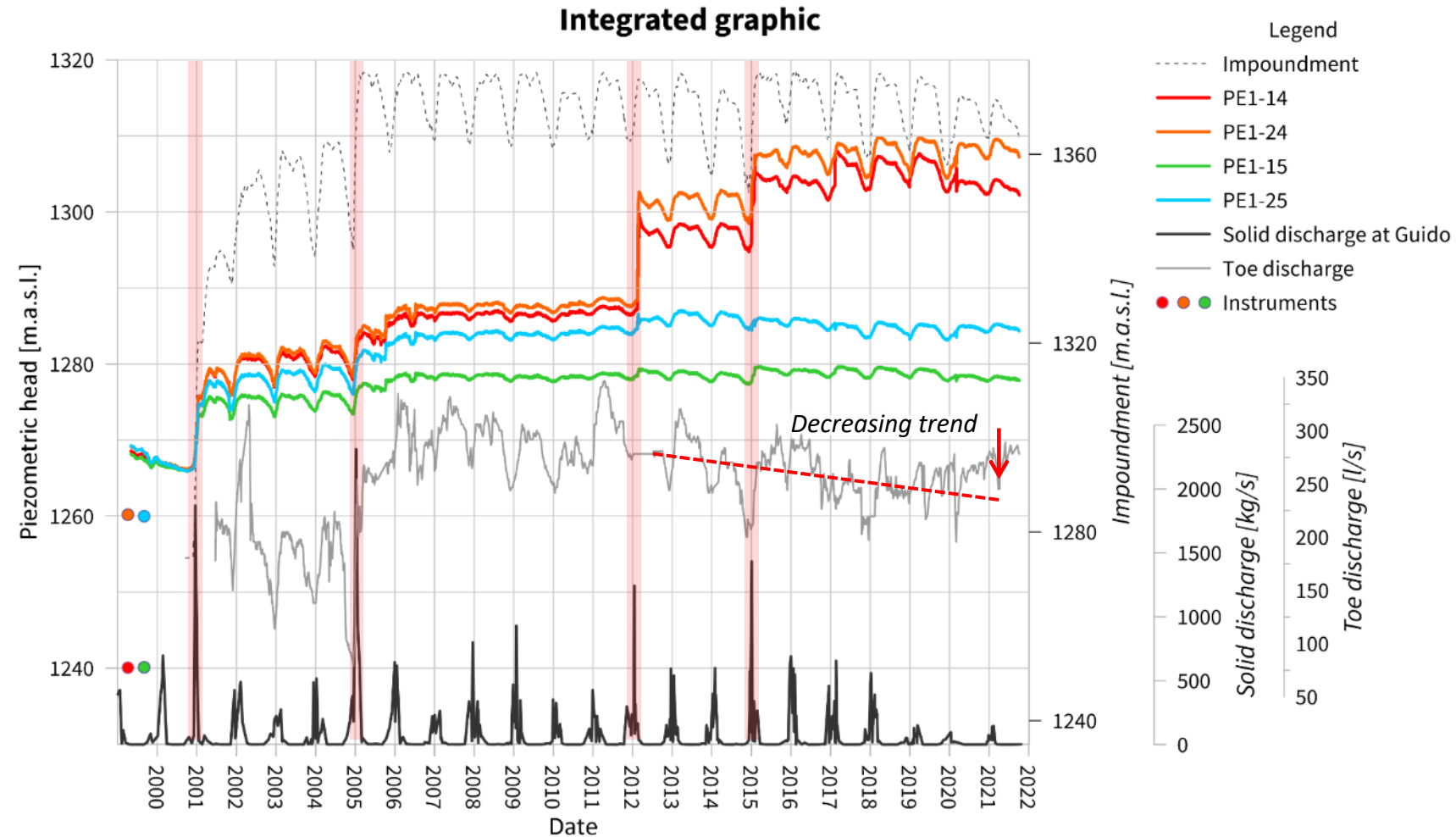
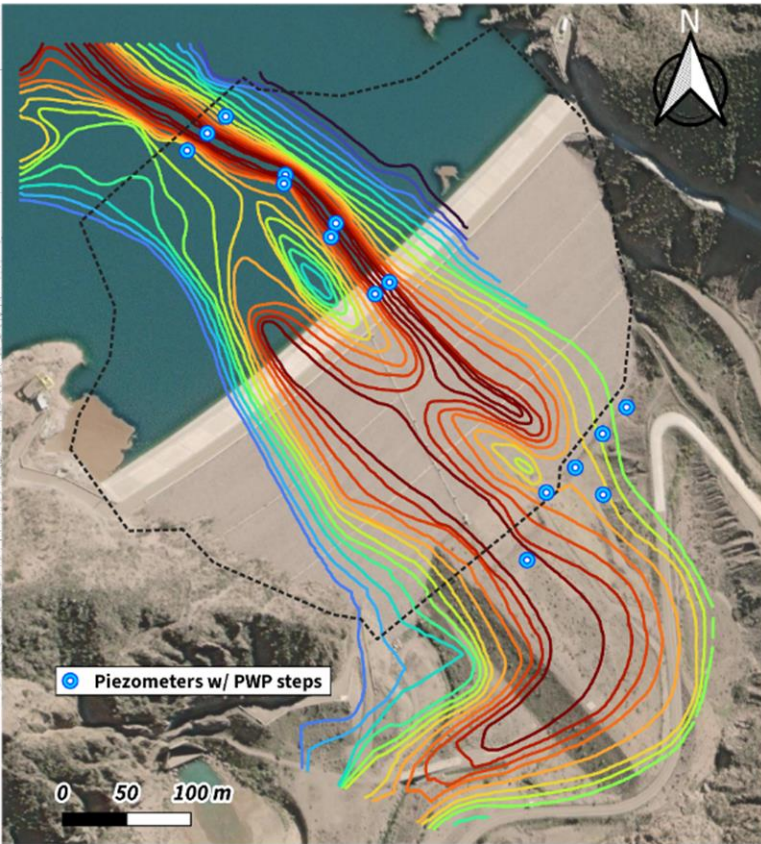
Instruments within the cut-off wall



Buried instruments

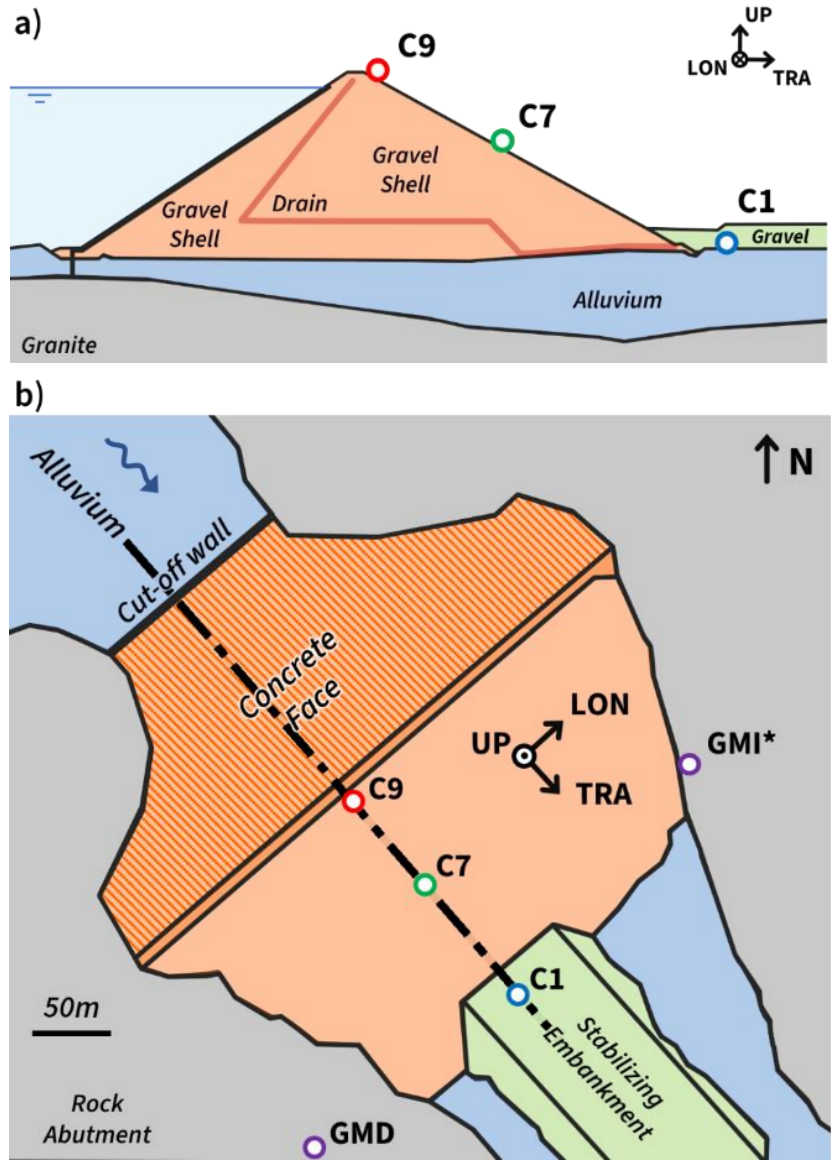


Piezometers with PWP steps aligned in the left-margin

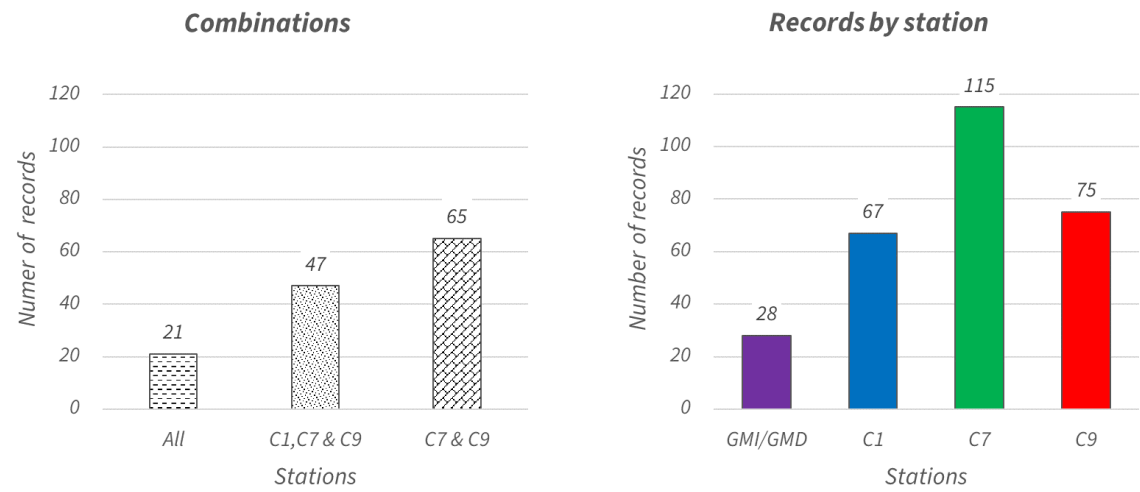


Likely reason: non-uniform change in the permeability due intrusion of fines from solid discharge in the alluvium

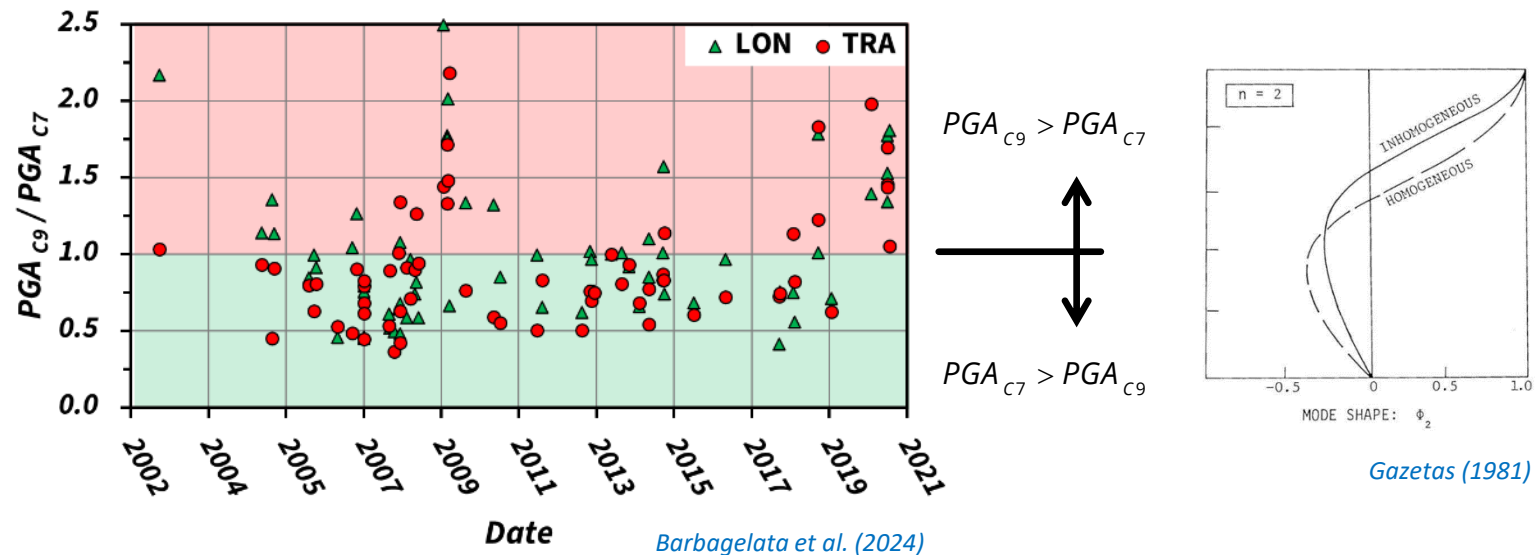
Seismic monitoring system: Instrument location and components

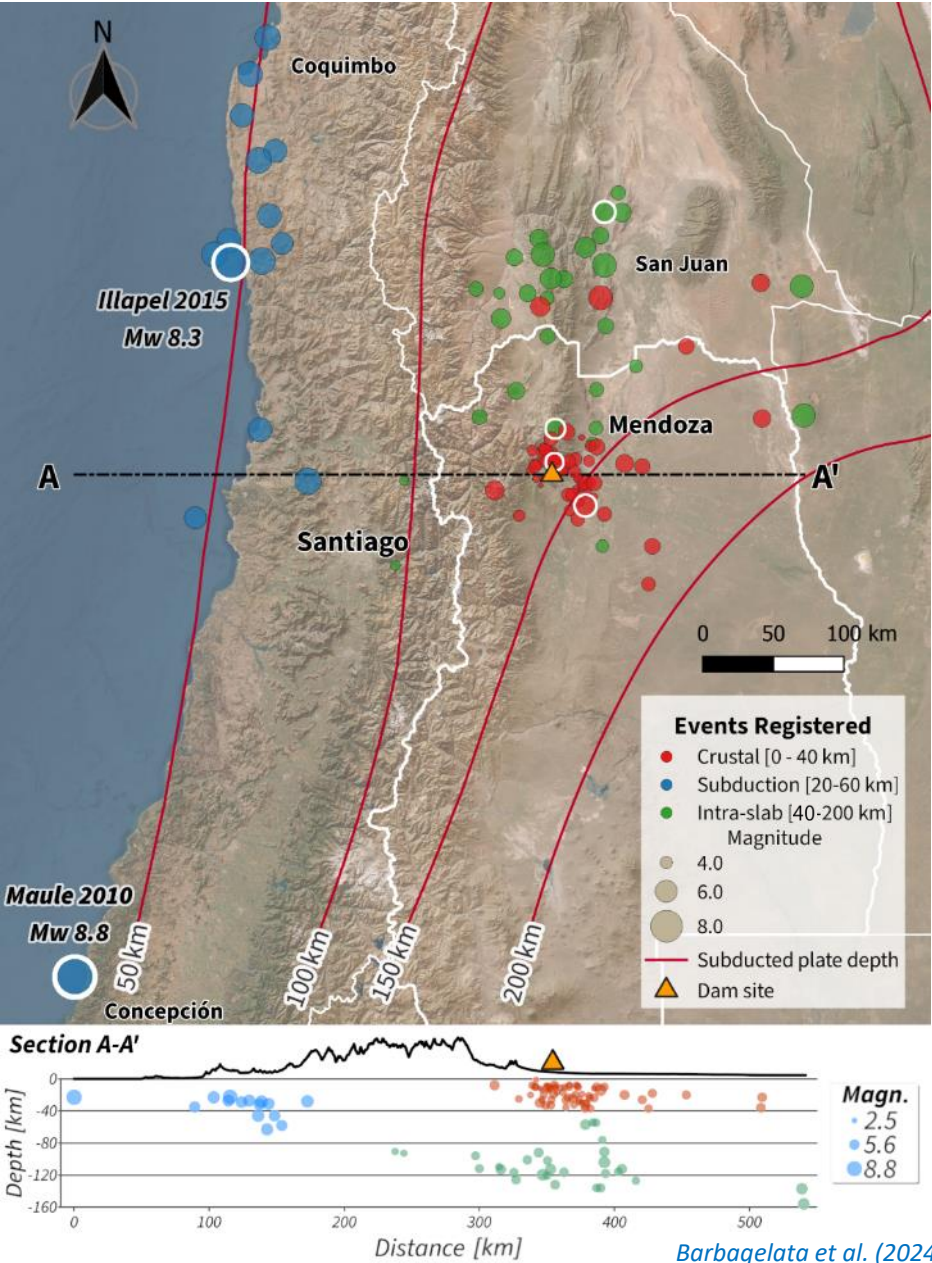


Records by station and combinations



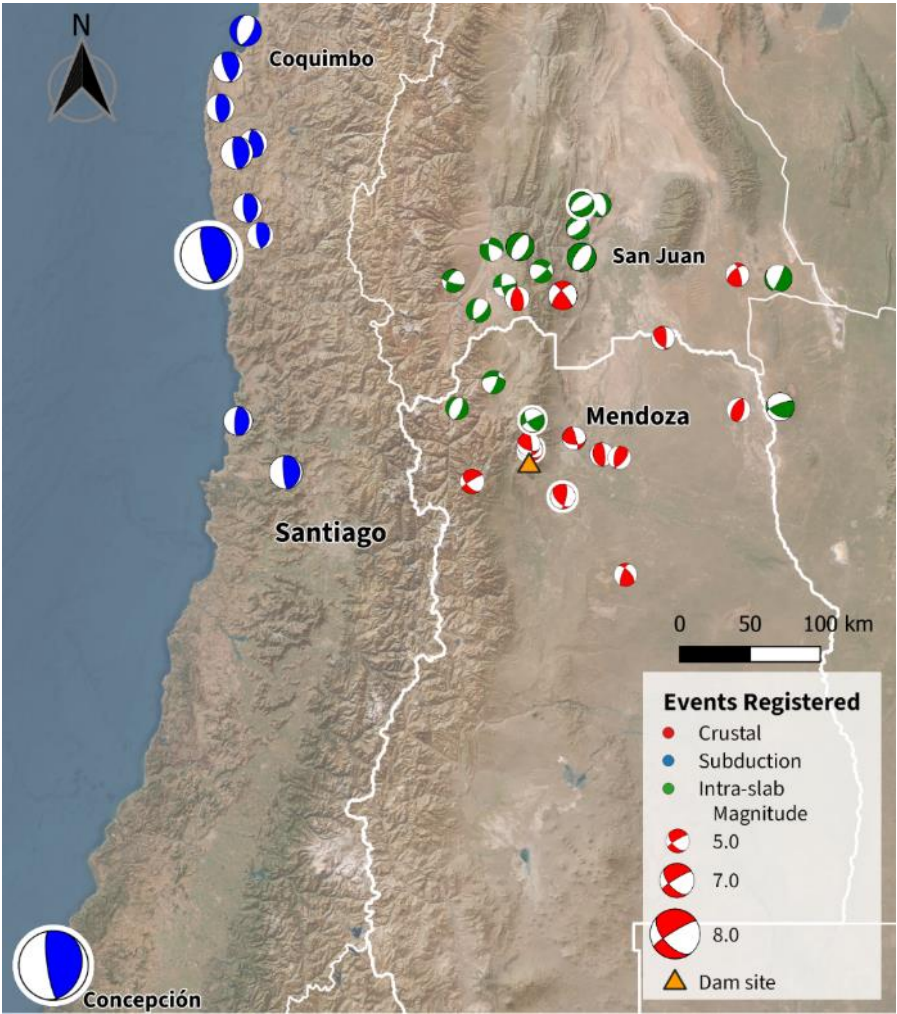
Ratio of peak acceleration at C9 and C7 for TRA and LON components



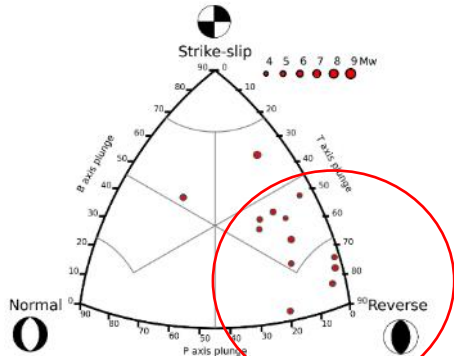


Location and characteristics of the weak-motions events recorded by the seismic monitoring system of the dam

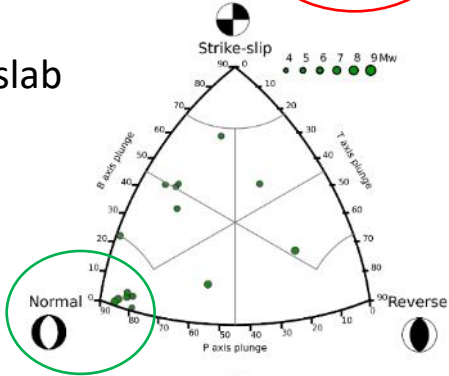
Focal mechanism for the events recorded



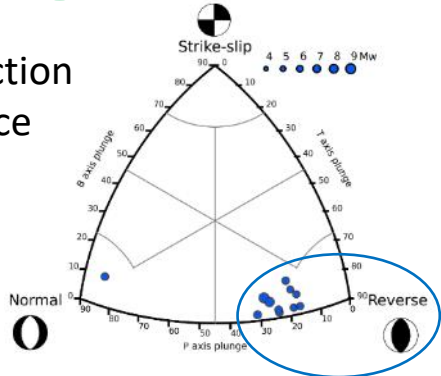
Crustal

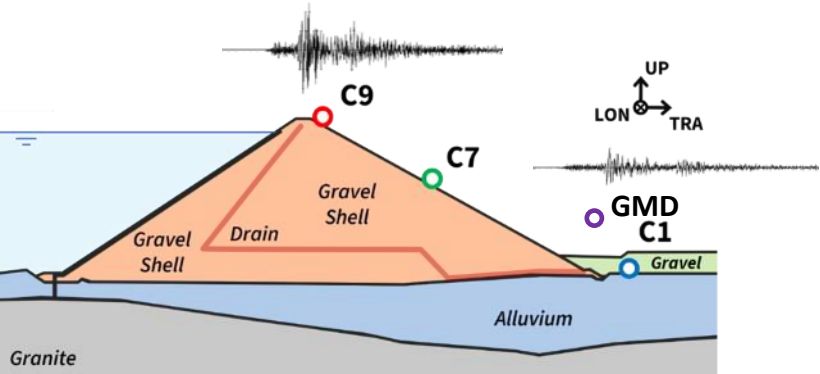


Intra-slab



Subduction Interface



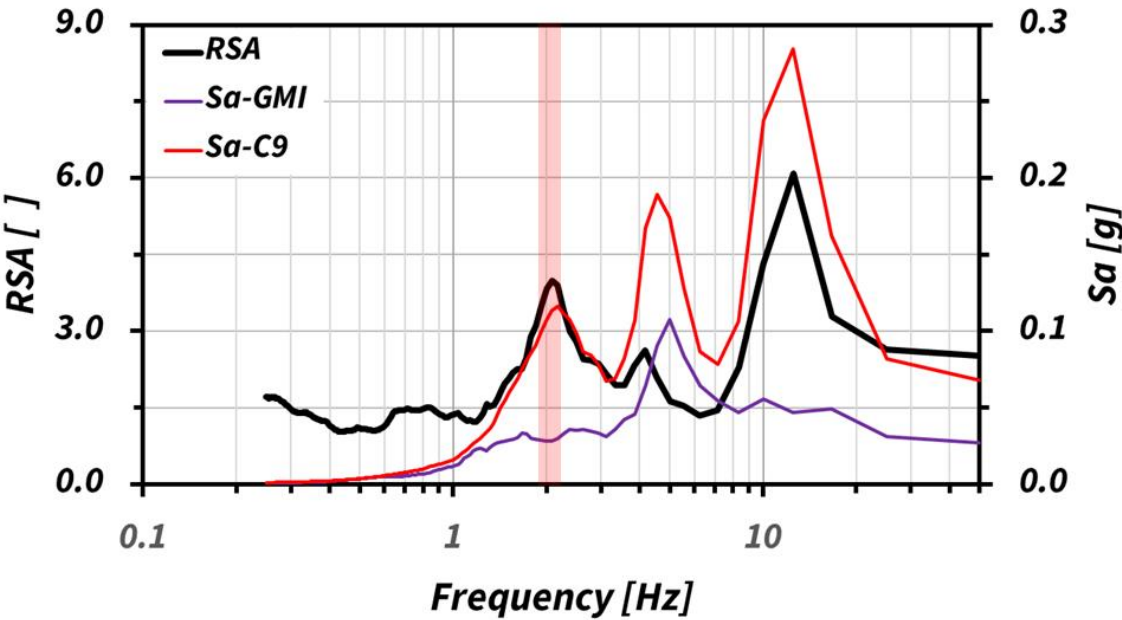


Selected events for record processing

Event	Date	magn. [Mw]	ep.dist. [km]	depth [km]	PGA C9 [g]	AI C9 [m/s]	f _D GMD [Hz]
CR-01	5/8/2006	5.6	33.7	32.0	0.065	0.0379	5.0
CR-02	23/6/2021	4.6	12.0	19.0	0.094	0.0283	10.0
IS-01	26/4/2009	5.1	33.9	132.0	0.007	0.0003	7.1
IS-02	18/1/2010	5.5	199.6	91.0	0.027	0.0050	3.6
SD-01	27/2/2010	8.8	502.3	22.9	0.024	0.0533	2.3
SD-02	16/9/2015	8.3	287.1	22.4	0.016	0.0326	4.2

Barbagelata et al. (2024)

Ratio of Spectral Acceleration (RSA)



Barbagelata et al. (2024)

$$SSR(f) = \frac{A(f)_{C9}}{A(f)_{GMD}}$$

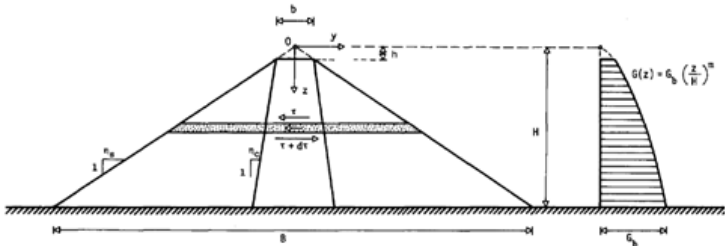
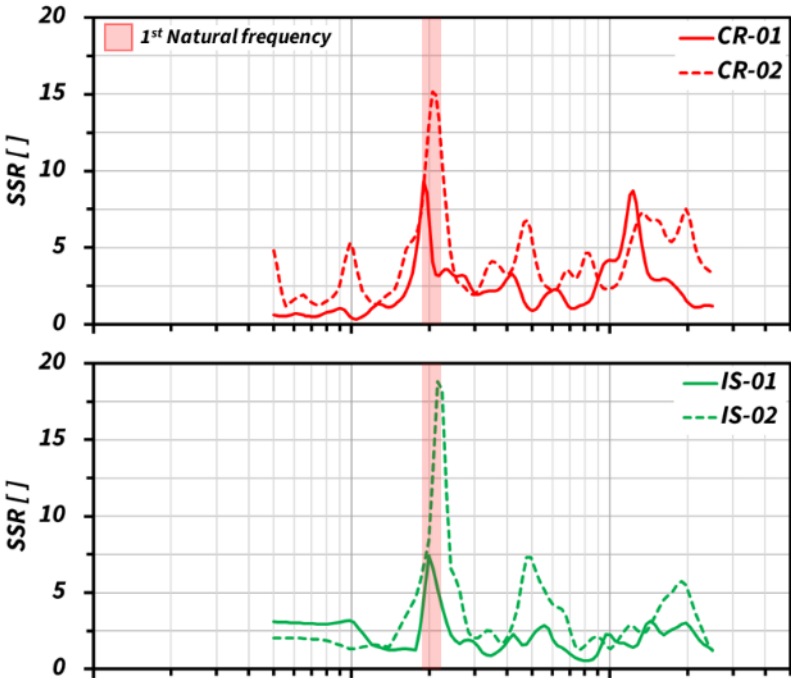
$$RSA(f) = \frac{Sa(f)_{C9}}{Sa(f)_{GMD}}$$

$f_1 \cong 2.0 \text{ Hz}$

$$f_1 = \frac{V_s}{2.57 \times 0.9 \times H}$$

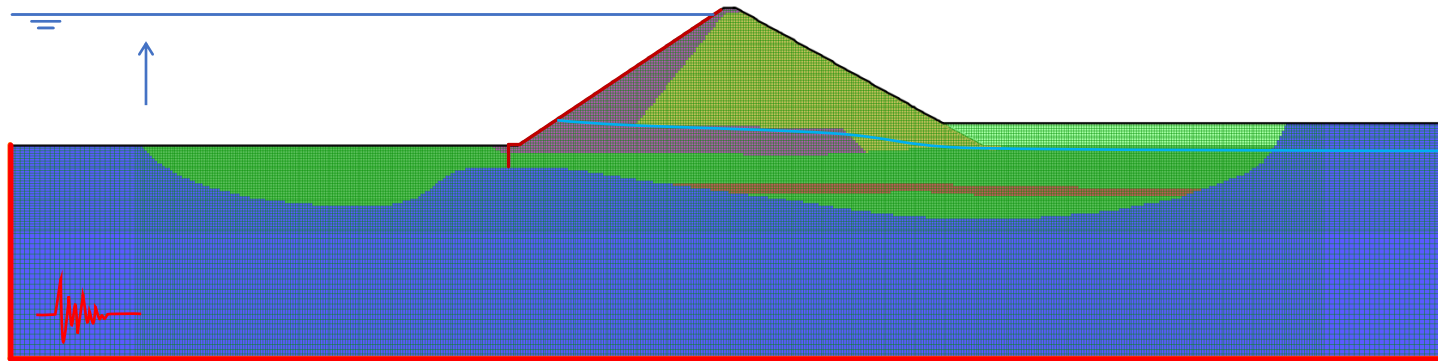
$V_s = 550 \text{ m/s}$

Standard Spectral Ratio (SSR)



Dakoulas & Gazetas (1985)

Finite Difference Numerical model (FLAC) – Section C



GEOMATERIALS	MATERIAL MODEL
Granite bedrock	Elastic
3A – Sand-Gravel-Pebble mixture	MC + Hyst. Damp.
3B – Sand-Gravel-Cobble mixture	MC + Hyst. Damp.
Foundation gravelly alluvium	PM4Sand
Sand/Silty sand alluvial material	PM4Sand
Stabilizing embankment	MC + Hyst. Damp.
/ RC Slab / Cut-off wall	Elastic / MC

1. Definition of the **geometry** of the regions and model **discretization**
2. Designation of **mechanical and hydraulic properties** for every region based on laboratory and in-situ tests
3. Setting up of mechanic and hydraulic **boundary conditions**
4. Reproduction of the **construction sequence** and calibration
5. Gradual increase of the **reservoir level**
6. Set the **dynamic properties** and **input motions**
7. **Running** the analyses
8. **Evaluating** the **results** and comparison with simplified analysis

Key points when running NDAs

- The model should **capture** the **response** of the dam and be **representative** of the selected Potential Failure Modes (PFMs)
- The material models can reproduce the complex behavior of the soils under seismic actions
- The selected Engineering Demand Parameters (EDPs) are informative about the damage condition given for every PFM

ICOLD B-166 criteria to start the inspections following earthquakes

Magnitude	Distance (km)
> 4.0	25
> 5.0	50
> 6.0	80
> 7.0	125
> 8.0	200

Or $MMI \geq 4$

ICOLD B-166 (2016)

Emergency Action Plan Initiation

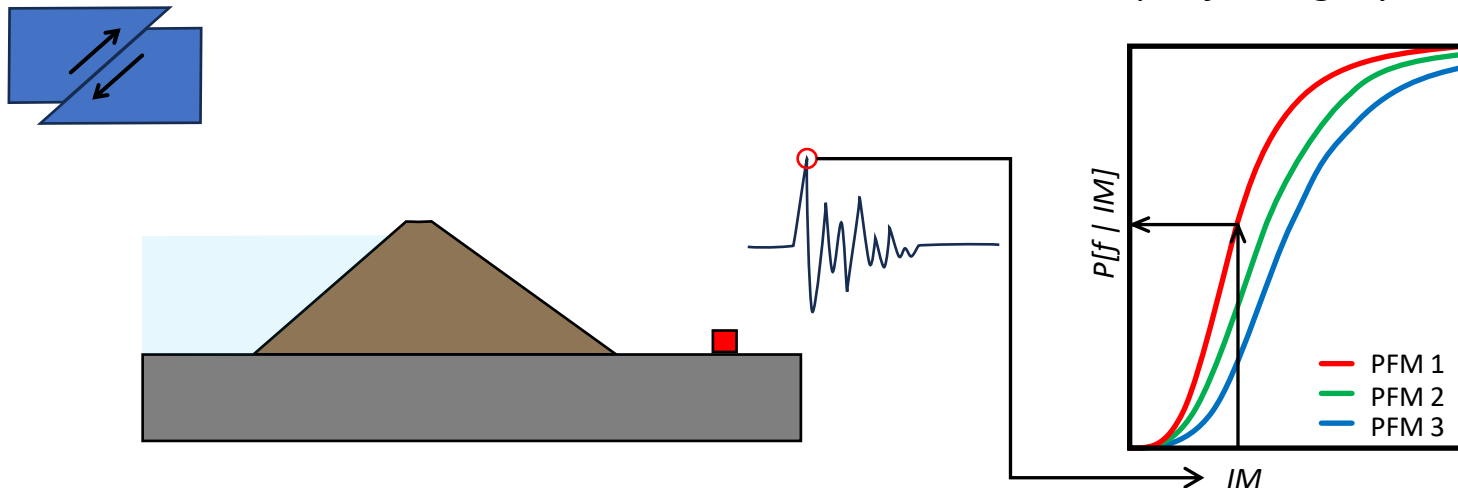
- Visual inspections of dam, appurtenant and reservoir
- Increase of recording rates of the instruments
- Evaluation of the evolution of the concerning features
- Communication with authorities and specialists

Is the dam at risk? Beginning or not of countermeasures

Can we bring more information about the dam performance, for a better decision making?

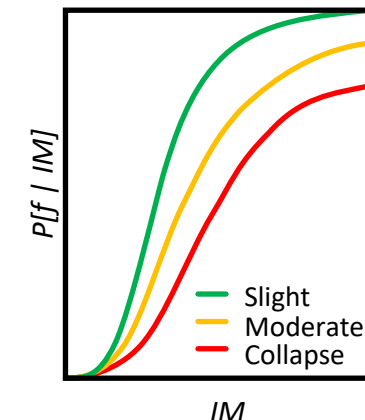
Groups of fragility functions

Dam-specific Fragility Functions

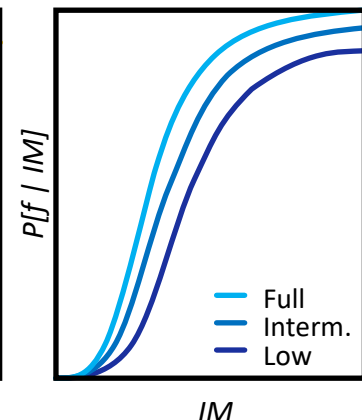


Information about the dam performance

Damage level



Reservoir level variation



- ❑ A **geotechnical model** representative of the **dam-subsoil system** that **integrates** all the information available (geological surveys, in-situ and laboratory geotechnical tests, analysis of monitoring records) is **essential** to describe the **complexity** of the dam and **site conditions**
- ❑ A **numerical model** able to **capture** the complex response and selected **Potential Failure Modes** of the dam is **decisive** in the **assessment** of the **performance** and **fragility** of the dam during strong ground motions
- ❑ **Evaluating the seismicity** and the **dynamic dam response** using the records from the **monitoring system** supports the **calibration** and validation of the **numerical dam-subsoil model**
- ❑ Coupling the **seismic monitoring** data records with the **fragility functions** is a promising innovative approach that could help the managers of infrastructure to take **safer decisions** during **post-event Emergency Actions Plans** after **seismic events**, moving towards the use of digital twins for dam safety in seismic regions



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Question time...

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Potreriillos dam and reservoir, July 2022